Piled Raft Foundations: Comparative Approaches and Soil Structure Interaction

Harsha Tadavarthi
Geotechnical Team Leader & Technical Manager | MIDAS IT



01 Soil Structure Interaction

- SSI
- Various SSI Methods
- Soil Continuum Method

02 Pile Raft Foundation

- Interactions
- Different ways to Model Piles & Raft

03 Demonstration

Different procedures for Pile Raft Modeling

04 Summary and Conclusion

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SSI

What is SSI?

Interaction of Stiffness and Deformationbetween Structure and Soil

Necessary for Adequate Assessment ofStresses and Forces in the SupportingStructure

• Why SSI?

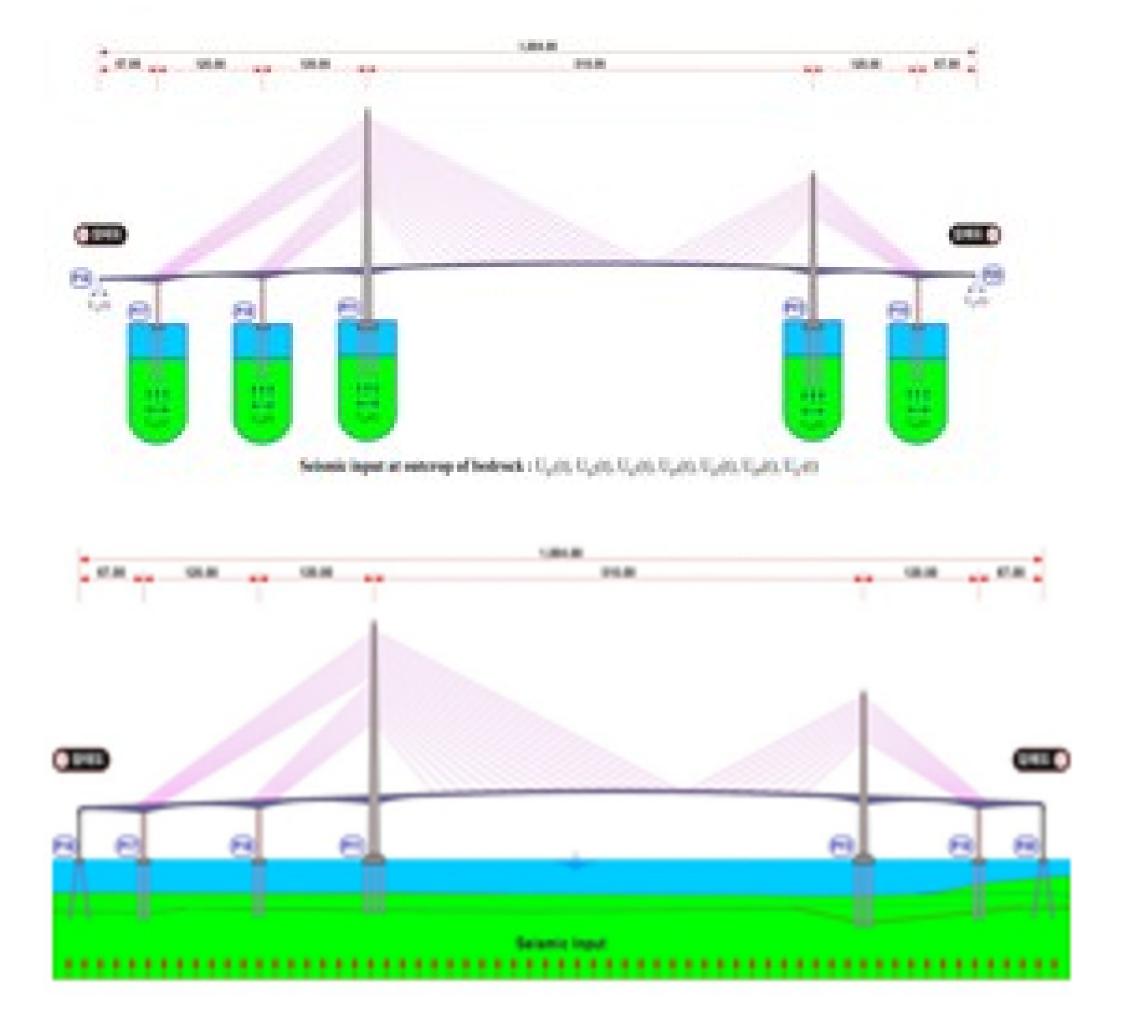
Supporting Soil,

- -Generates Loading and
- –Provides Resistance to Loading

Force on Deck and Pier depends on,

- –Location of the foundation
- –Flexibility of foundation
- -Supporting Soil Behaviour

Broad Classification of SSI Methods



Substructure Method

Based on superposition of events, it separates the problem into two simpler parts.

- Free Field Analysis: The reaction / response of the soil is determined (mainly where the structure will be)
- Structural Analysis: The soil can be modeled as spring damper system(impedance) with that response. The detailed structure is designed with the idealization of soil as independent damper spring
 Eg: Wrinkler Springs, Springs from Empirical Equations etc

Direct Method

The soil-structure system is modeled and analyzed in one step directly Get response with the two simultaneously

Numerical methods: Continuum Methods FEM, FDM

Various SSI Methods

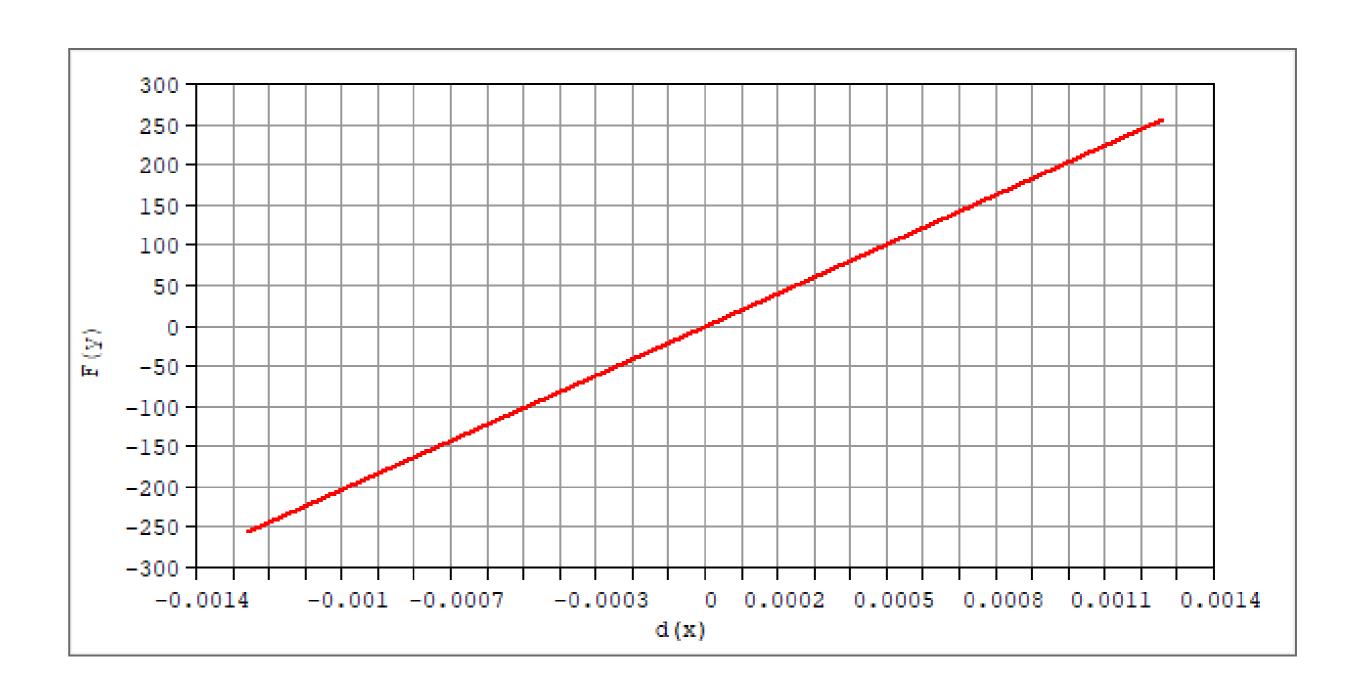
Linear Springs

Shallow Foundations:

- Winkler Method
- Terzaghi (1955)
- Bowles
- Vesic (1961, 1973)
- Poulos & Davis (1974)
- Gazetas (1991)
- Kausel & Roesset (1975)

Piles:

- Randolph & Wroth (1978)
- Vesic (1977)
- Poulos & Davis (1974)
- NAVFAC DM-7



Linear

Comp.-only

Tens.-only

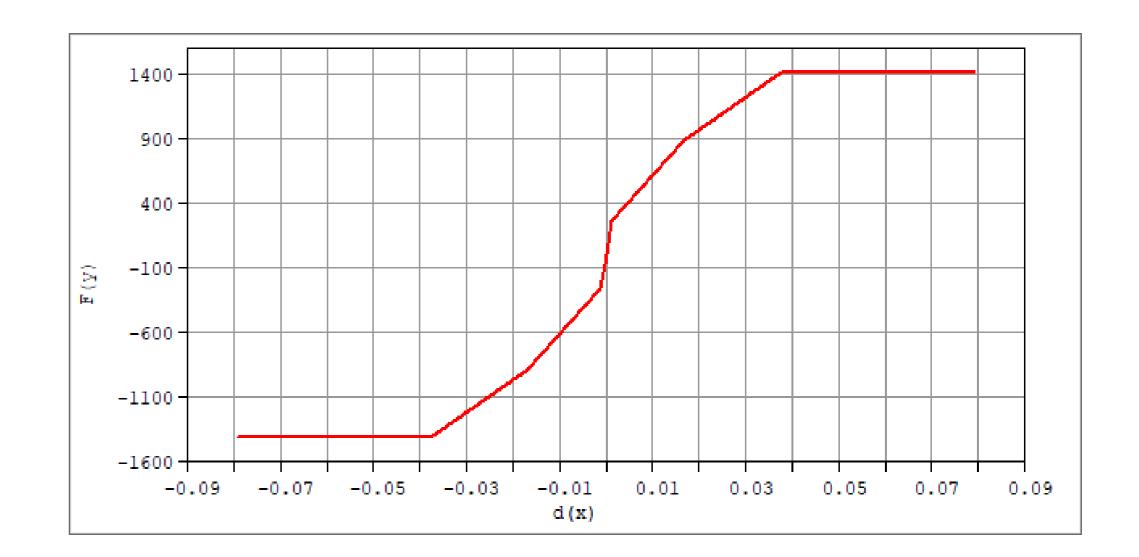
Multi Linear or Nonlinear

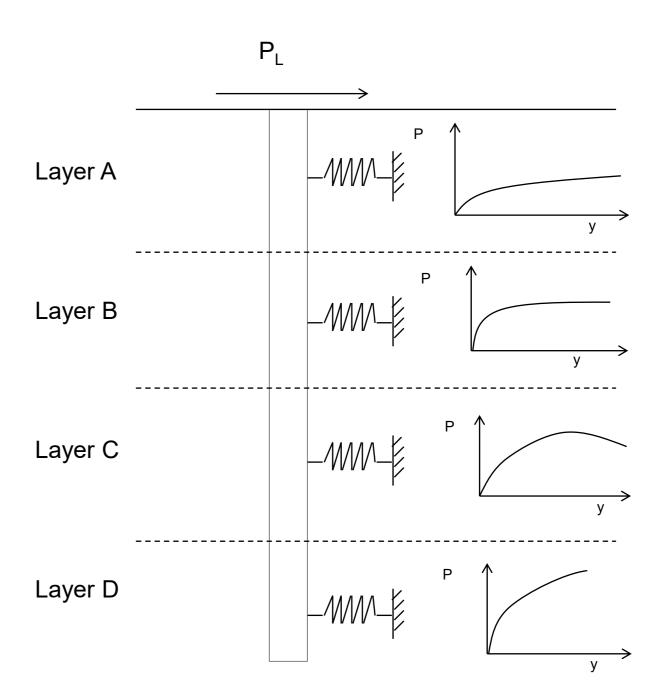
Shallow Foundations:

- Meyerhof (1965, 1967)
- Vesic (1973)
- Pressure–settlement curves from plate load tests – inherently nonlinear.
- Code-based nonlinear ks correlations (e.g., IS 2950, Eurocode 7, AASHTO with modulus variation).

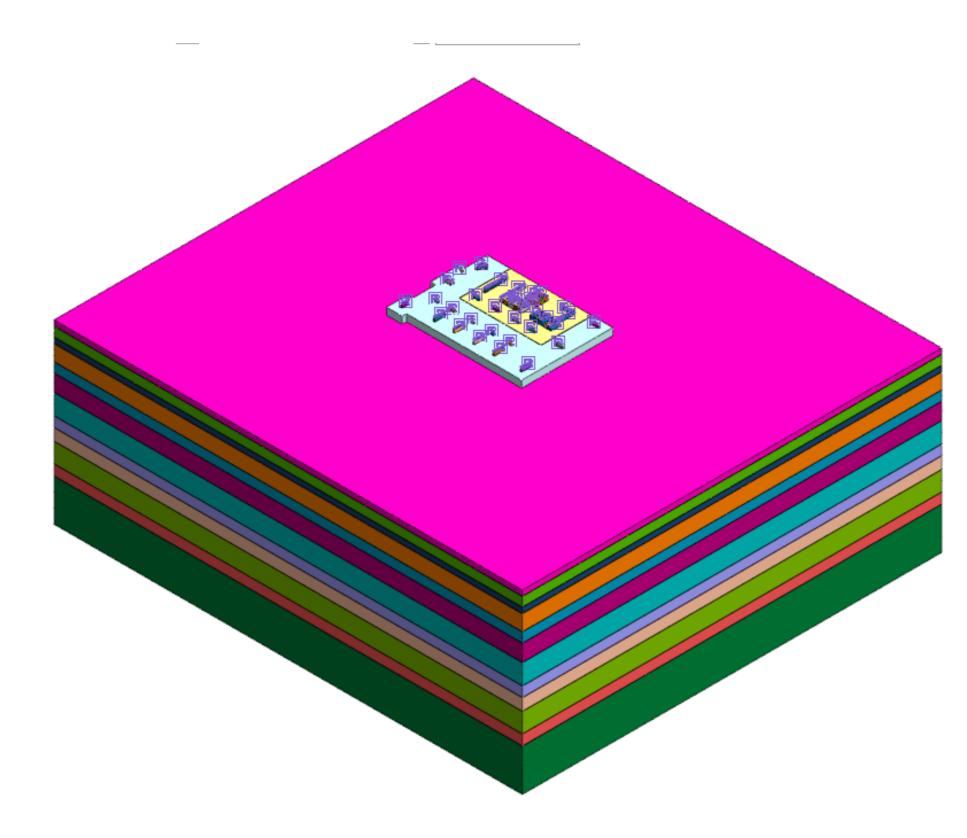
Piles:

- Matlock (1970)
- Reese, Cox & Koop (1974)
- Reese & Matlock (1974)
- Coyle & Reese (1966)
- Vesic (1977)
- Reese & O'Neill (1987)
- API RP 2A / API RP 2GEO
- DNVGL-RP-C212
- FHWA NHI-05-046

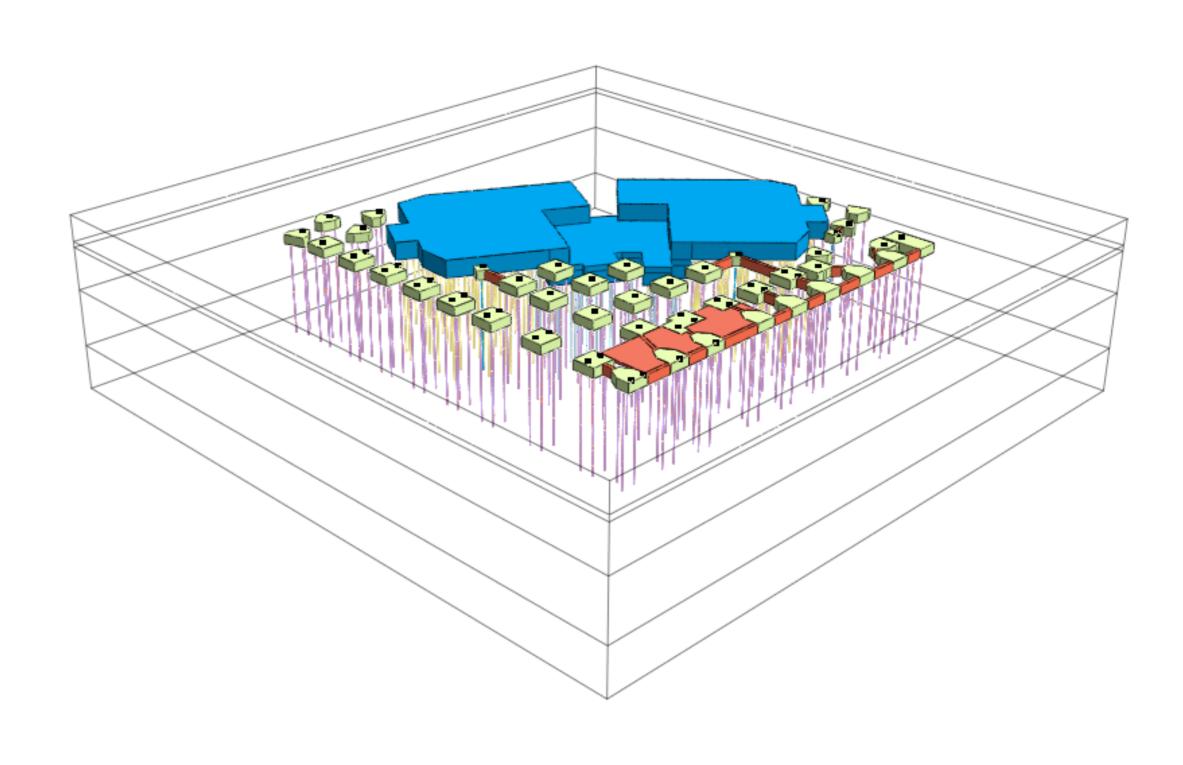




Soil Continuum Method

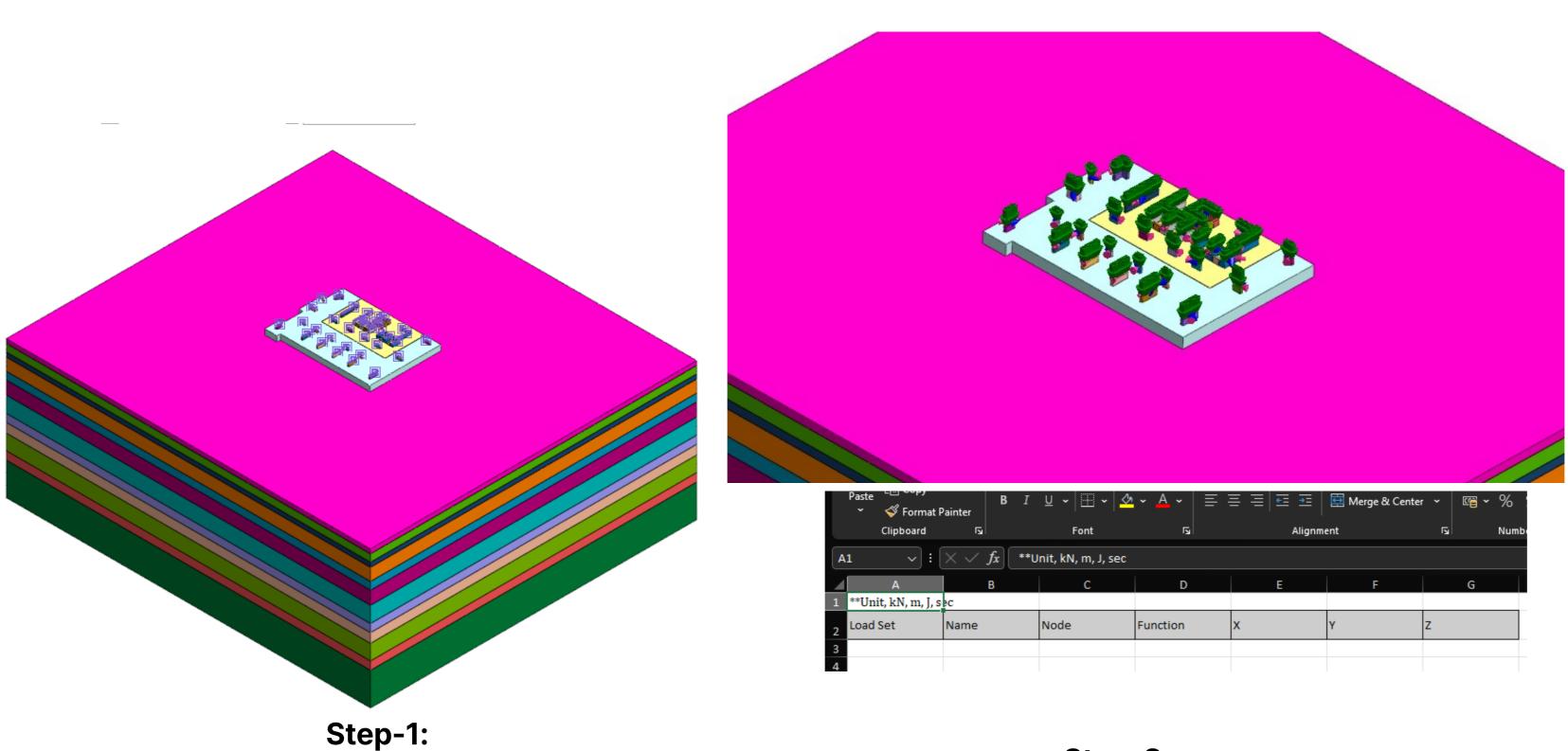


Raft Foundation - Soil Continuum Method



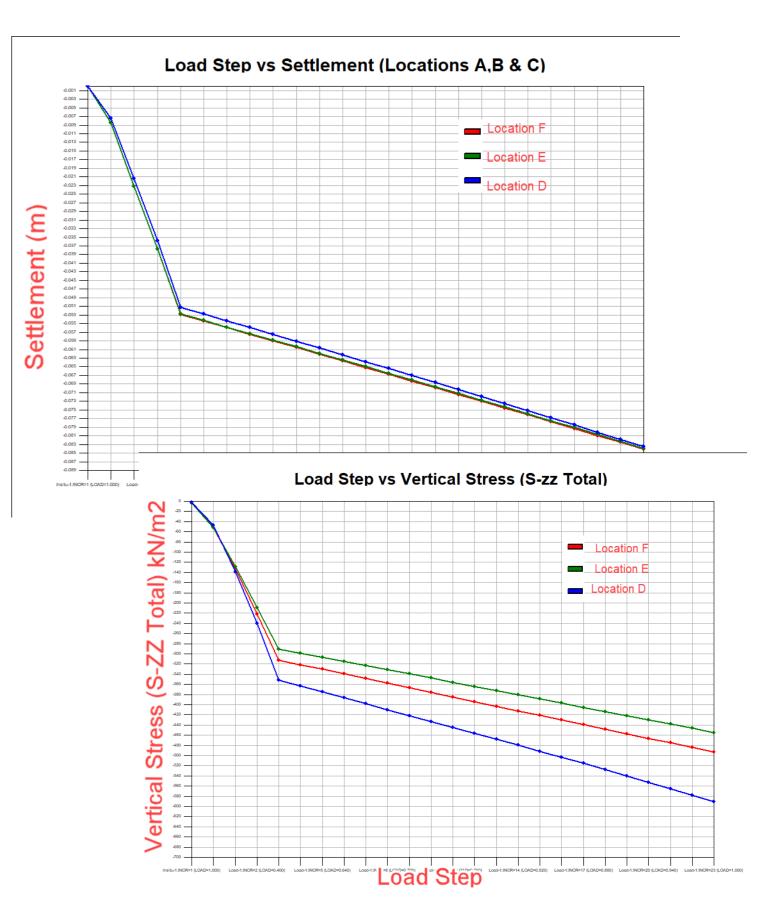
Pile Raft Foundation - Soil Continuum Method

Soil Continuum Method – Application Procedure



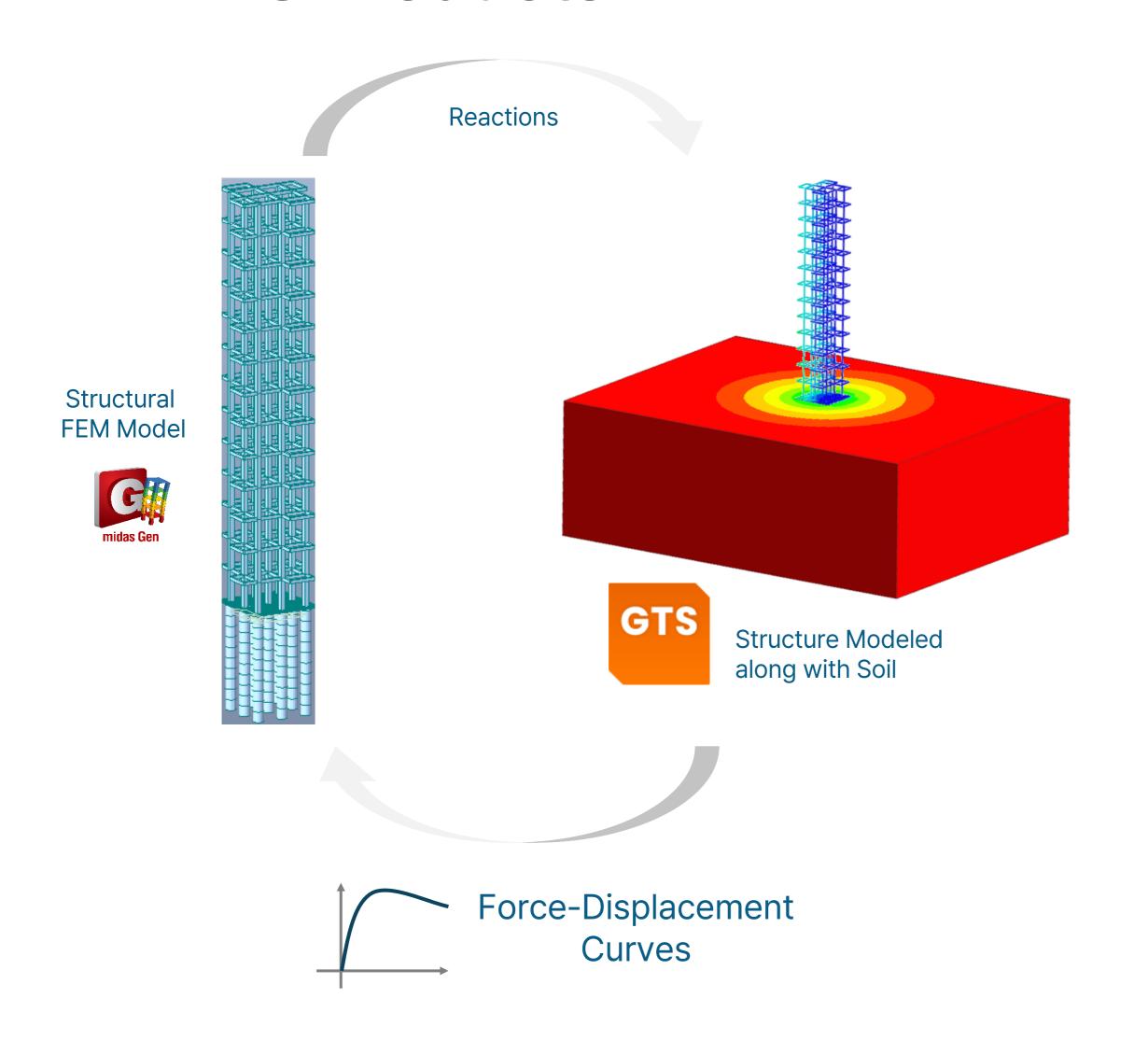
Foundation and Soil Modeling

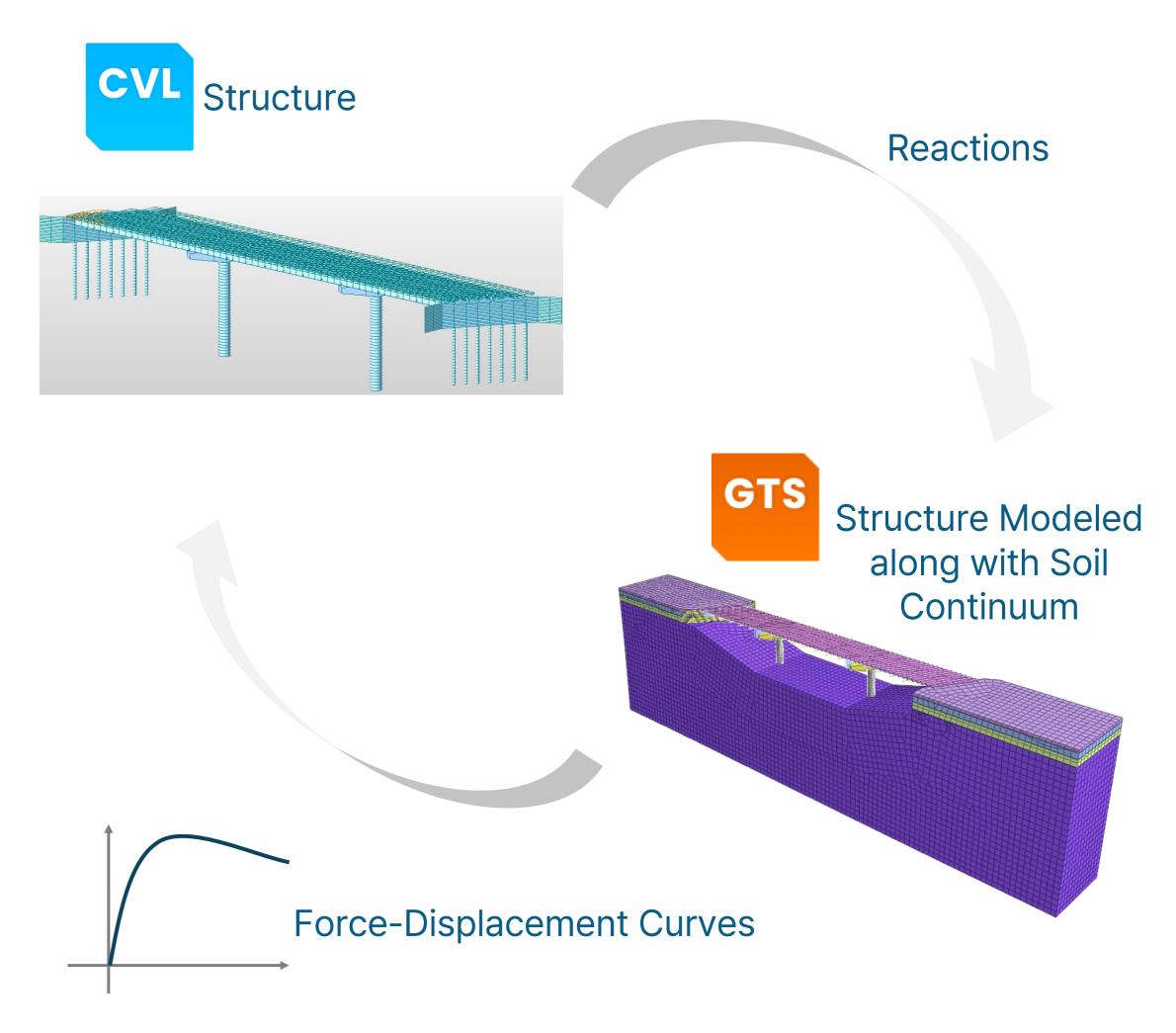
Step-2:
Load Table Import/Export Option.
(Load imported into GTS NX via excel sheet from any Structural tool)



Step-3: Export the Stiffnesses back to Structural tool

Soil Continuum Method – Application Procedure – Fully Automated in MIDAS Products





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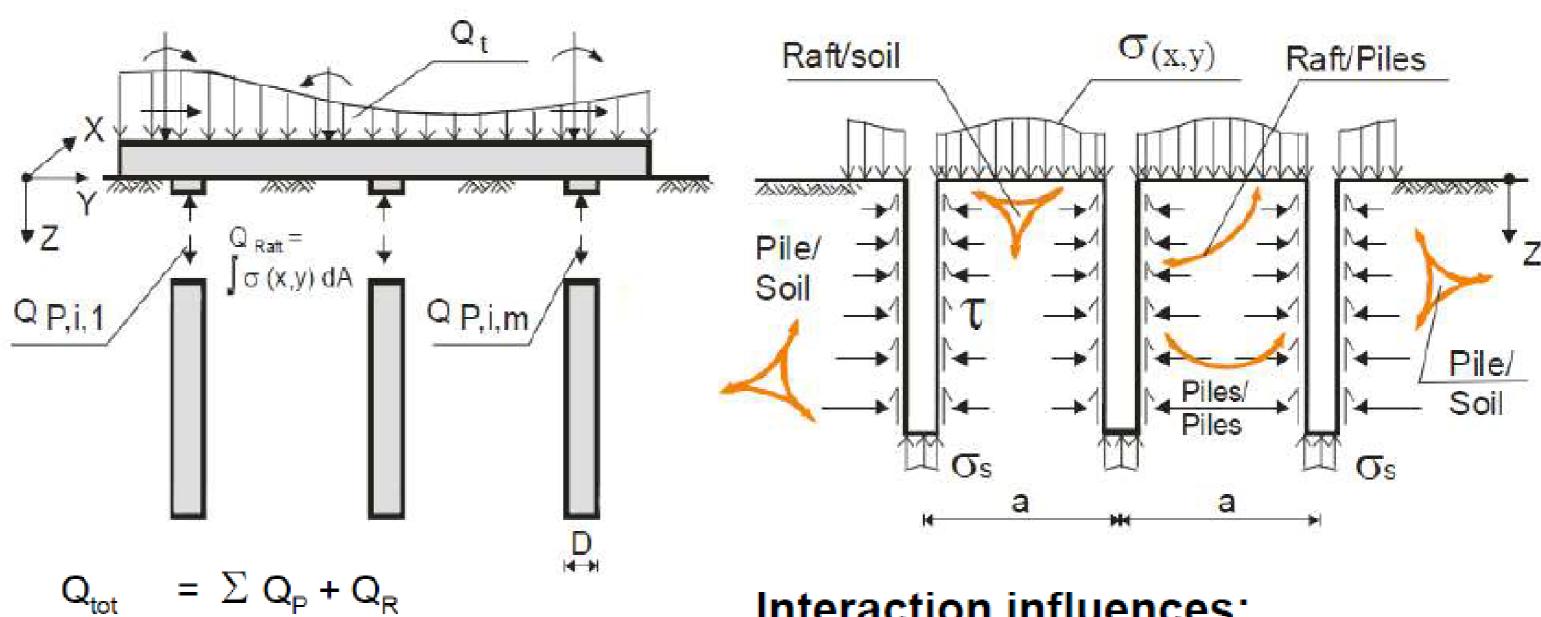
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Pile Raft Foundation

Interactions



$$Q_p = Q_b + Q_s$$

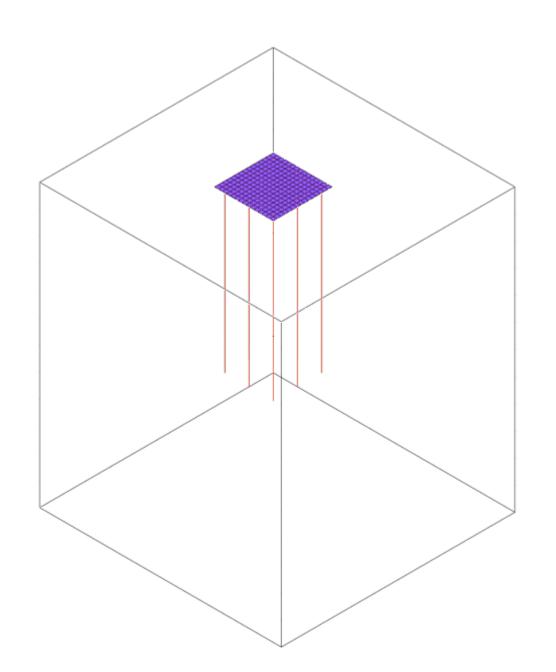
$$Q_R = \int \sigma(x,y) dA$$

$$\textbf{Q}_{\text{tot}} \quad \geq \ \boldsymbol{\eta} \cdot \boldsymbol{\Sigma} \ \textbf{S}_{\text{tot}}$$

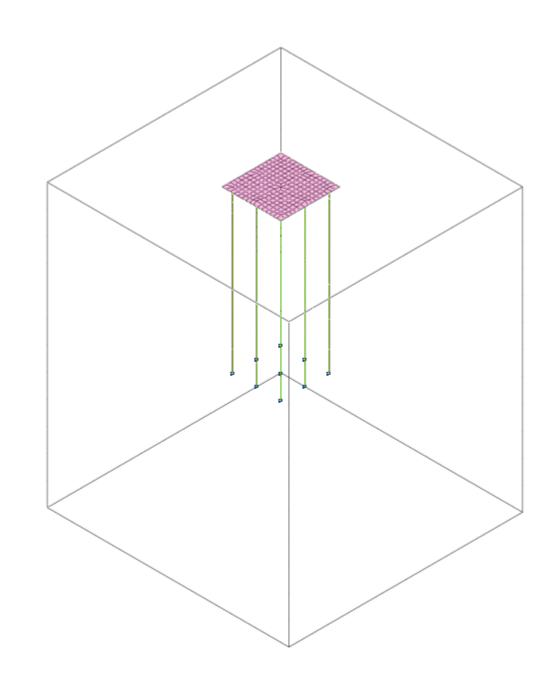
Interaction influences:

- Pile-Soil interaction
- Pile-Pile interaction
- Raft-Soil interaction
- Pile-Raft interaction

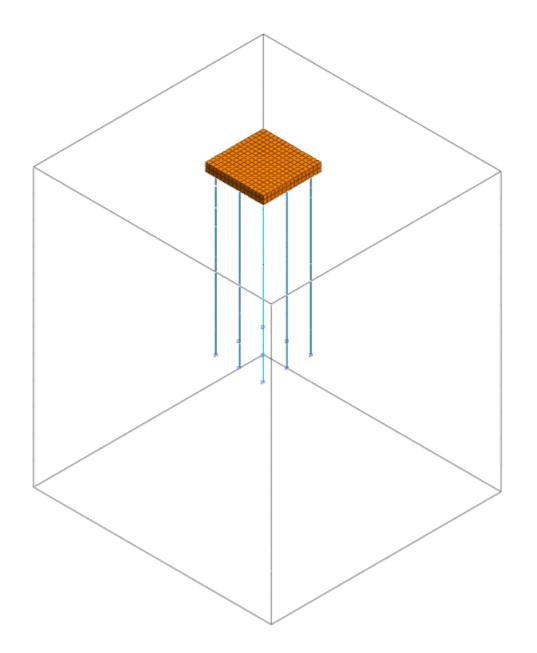
Different Ways to Model Pile Raft Foundation



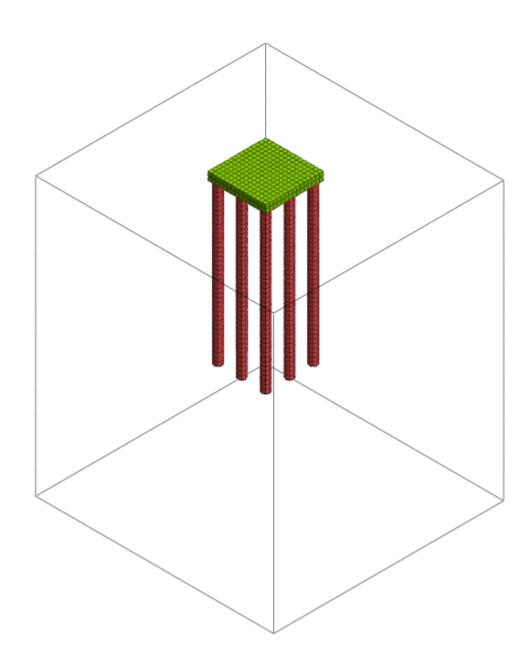
- Pile as Embedded Beam
- Raft as Shell Element



- Pile as Beam element with Skin
 Friction and End Bearing Definitions
- Raft as Shell Element



- Pile as Beam element with Skin Friction and End Bearing Definitions
- Raft as Solid Element



- Pile as Volumetric Element and Plane Interface is use for Skin Friction Definition
- Raft as Solid Element

In General,

- If Pile is slender (L/D > 10-15), then Modeling it as Beam is preferred. For Non-Slender or Short Pile/Large
 diameter such as caissons, drilled shafts, Volumetric Pile is preferred
- Raft thickness is small compared to length and breadth; Shell is Preferred. If Raft is thick (t>~1/5 to 1/10 of length), solid raft is preferred.

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Let's Model!!

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Conclusion

- Though there are multiple method for modeling of the pile raft foundation, the results may not be the same in all the methods.
- The SSI and the stress distribution may not be the same in all the methods.
- Shell and beam elements are preferred for quick assessment; Solids are preferred for detailed assessment.
- It is always necessary to calibrate the inputs with the field tests. Running a Back Analysis or Soil Test features of GTS NX would help in Calibration.

(The recent version of GTS NX offers Back Analysis/Optimization analysis that can help users to optimize the inputs for the given field test data)

Thanks for Attending!!

